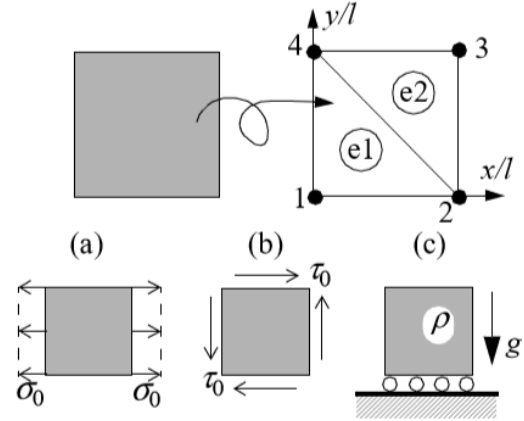


Tutorial 4: FEM for Engineering Applications (SE1025)

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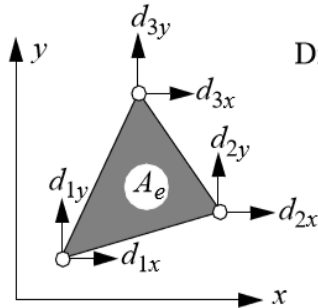
6.14 Consider a thin quadratic sheet metal of size $l \times l$ and thickness h of a linear elastic material (E , ν). Model the sheet metal by use of two linear triangular elements (CST-element) and carry out FEM analyses for the three different load cases (a), (b) and (c). Introduce appropriate displacement boundary conditions, where symmetry conditions can be utilized, and determine node displacements and stresses in the elements. For simplicity, let Poisson's ratio be $\nu = 0$.

- Load cases: (a) uniaxial tension,
 (b) pure shear,
 (c) dead weight, where ρ is the density and g acceleration of gravity.



FORMULAS

Plane (2D) triangular linear element:



Displacements:

$$\begin{bmatrix} u(x, y) \\ v(x, y) \end{bmatrix} = \begin{bmatrix} N_1 & 0 & N_2 & 0 & N_3 & 0 \\ 0 & N_1 & 0 & N_2 & 0 & N_3 \end{bmatrix} \mathbf{d}_e = \mathbf{N} \mathbf{d}_e$$

$$\mathbf{d}_e = \begin{bmatrix} d_{1x} \\ d_{1y} \\ d_{2x} \\ d_{2y} \\ d_{3x} \\ d_{3y} \end{bmatrix}$$

$$N_1 = \frac{1}{2A_e} [(y_2 - y_3)(x - x_2) + (x_3 - x_2)(y - y_2)]$$

$$N_2 = \frac{1}{2A_e} [(y_3 - y_1)(x - x_3) + (x_1 - x_3)(y - y_3)]$$

$$N_3 = \frac{1}{2A_e} [(y_1 - y_2)(x - x_1) + (x_2 - x_1)(y - y_1)]$$

Strains:

$$\begin{bmatrix} \epsilon_{xx} \\ \epsilon_{yy} \\ \gamma_{xy} \end{bmatrix} = \mathbf{B} \mathbf{d}_e \quad \mathbf{B} = [\mathbf{B}_1 \quad \mathbf{B}_2 \quad \mathbf{B}_3] \quad \mathbf{B}_i = \begin{bmatrix} \partial N_i / \partial x & 0 \\ 0 & \partial N_i / \partial y \\ \partial N_i / \partial y & \partial N_i / \partial x \end{bmatrix}$$

Stresses:

$$\begin{bmatrix} \sigma_{xx} \\ \sigma_{yy} \\ \sigma_{xy} \end{bmatrix} = \frac{E}{(1 - \nu^2)} \begin{bmatrix} 1 & \nu & 0 \\ \nu & 1 & 0 \\ 0 & 0 & (1 - \nu)/2 \end{bmatrix} \begin{bmatrix} \epsilon_{xx} \\ \epsilon_{yy} \\ \gamma_{xy} \end{bmatrix} \quad (\text{Plane stress})$$

Solution

Theory Recall

The FEM equilibrium equation for the element is:

$$\int_{V_e} \mathbf{B}^T \mathbf{C} \mathbf{B} dV \mathbf{d}_e = \int_{S_e} \mathbf{N}^T \mathbf{t} dS + \int_{V_e} \mathbf{N}^T \mathbf{K} dV$$

The displacement field is given by:

$$\begin{bmatrix} u(x, y) \\ v(x, y) \end{bmatrix} = \begin{bmatrix} N_1 & 0 & N_2 & 0 & N_3 & 0 \\ 0 & N_1 & 0 & N_2 & 0 & N_3 \end{bmatrix} \begin{bmatrix} d_{1x} \\ d_{1y} \\ d_{2x} \\ d_{2y} \\ d_{3x} \\ d_{3y} \end{bmatrix}$$

The shape functions are:

$$\begin{aligned} N_1(x, y) &= \frac{1}{2A_e} [(y_2 - y_3)(x - x_2) + (x_3 - x_2)(y - y_2)] \\ N_2(x, y) &= \frac{1}{2A_e} [(y_3 - y_1)(x - x_3) + (x_1 - x_3)(y - y_3)] \\ N_3(x, y) &= \frac{1}{2A_e} [(y_1 - y_2)(x - x_1) + (x_2 - x_1)(y - y_1)] \end{aligned}$$

In order to compute the Stiffness Matrix:

$$\int_{V_e} \mathbf{B}^T \mathbf{C} \mathbf{B} dV = \mathbf{K}_e$$

we need :

C: the constitutive matrix for isotropic material in plane stress condition

$$\mathbf{C} = \frac{E}{1 - \nu^2} \begin{bmatrix} 1 & \nu & 0 \\ \nu & 1 & 0 \\ 0 & 0 & \frac{1 - \nu}{2} \end{bmatrix}$$

B: the derivative of the shape function matrix

$$\mathbf{B} = \begin{bmatrix} \frac{\partial N_1}{\partial x} & 0 & \frac{\partial N_2}{\partial x} & 0 & \frac{\partial N_3}{\partial x} & 0 \\ 0 & \frac{\partial N_1}{\partial y} & 0 & \frac{\partial N_2}{\partial y} & 0 & \frac{\partial N_3}{\partial y} \\ \frac{\partial N_1}{\partial y} & \frac{\partial N_1}{\partial x} & \frac{\partial N_2}{\partial y} & \frac{\partial N_2}{\partial x} & \frac{\partial N_3}{\partial y} & \frac{\partial N_3}{\partial x} \end{bmatrix}$$

$$\mathbf{B} = \frac{1}{2A_e} \begin{bmatrix} (y_2 - y_3) & 0 & (y_3 - y_1) & 0 & (y_1 - y_2) & 0 \\ 0 & (x_3 - x_2) & 0 & (x_1 - x_3) & 0 & (x_2 - x_1) \\ (x_3 - x_2) & (y_2 - y_3) & (x_1 - x_3) & (y_3 - y_1) & (x_2 - x_1) & (y_1 - y_2) \end{bmatrix}$$

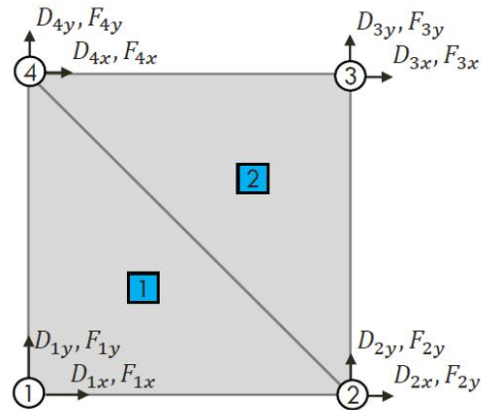
Our Problem:

2 st CST-element

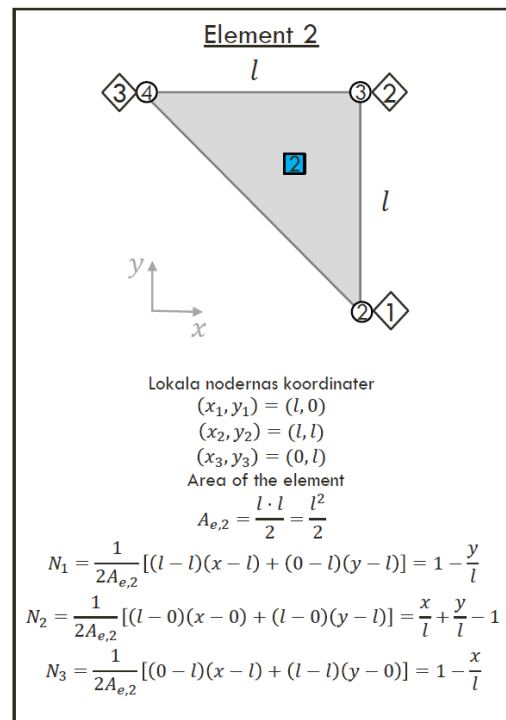
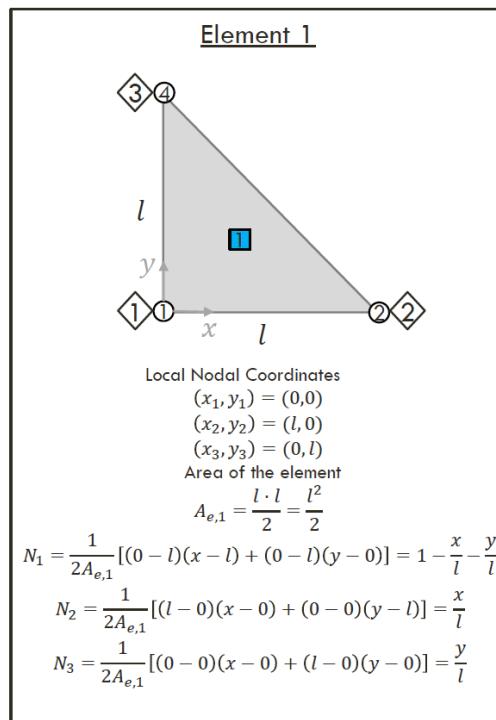
$$\mathbf{D}^T = [D_{1x} \ D_{1y} \ D_{2x} \ D_{2y} \ D_{3x} \ D_{3y} \ D_{4x} \ D_{4y}]$$

$$\mathbf{F}^T = [F_{1x} \ F_{1y} \ F_{2x} \ F_{2y} \ F_{3x} \ F_{3y} \ F_{4x} \ F_{4y}]$$

$$\mathbf{k}_e = \int_{V_e} \mathbf{B}^T \mathbf{C} \mathbf{B} dV \Rightarrow \mathbf{k}_e = \int_{A_e} h \mathbf{B}^T \mathbf{C} \mathbf{B} dA$$



We consider the elements:



◇ Local node ○ Global node

Constitutive behavior:

$$\mathbf{C} = \frac{E}{1-\nu^2} \begin{bmatrix} 1 & \nu & 0 \\ \nu & 1 & 0 \\ 0 & 0 & \frac{1-\nu}{2} \end{bmatrix} \Rightarrow \mathbf{C} = E \begin{bmatrix} 1 & 0 & 0 \\ 0 & 1 & 0 \\ 0 & 0 & \frac{1}{2} \end{bmatrix} \quad \boxed{\nu = 0}$$

Element 1:

$$\mathbf{k}_{e1} = \int_{A_e} h \mathbf{B}_{e1}^T \mathbf{C} \mathbf{B}_{e1} dA = h \int_{A_e} \underbrace{\mathbf{B}_{e1}^T \mathbf{C} \mathbf{B}_{e1}}_{\text{constant}} dA = A_1 h \mathbf{B}_{e1}^T \mathbf{C} \mathbf{B}_{e1} = \frac{l^2}{2} h \mathbf{B}_{e1}^T \mathbf{C} \mathbf{B}_{e1}$$

$$\Rightarrow \mathbf{B}_{e1} = \frac{l}{2Ae,1} \begin{bmatrix} -1 & 0 & 1 & 0 & 0 & 0 \\ 0 & -1 & 0 & 0 & 0 & 1 \\ -1 & -1 & 0 & 1 & 1 & 0 \end{bmatrix} = \frac{1}{l} \begin{bmatrix} -1 & 0 & 1 & 0 & 0 & 0 \\ 0 & -1 & 0 & 0 & 0 & 1 \\ -1 & -1 & 0 & 1 & 1 & 0 \end{bmatrix}$$

$$\mathbf{k}_{e1} = \frac{Eh}{4} \begin{bmatrix} 3 & 1 & -2 & -1 & -1 & 0 \\ 1 & 3 & 0 & -1 & -1 & -2 \\ -2 & 0 & 2 & 0 & 0 & 0 \\ -1 & -1 & 0 & 1 & 1 & 0 \\ -1 & -1 & 0 & 1 & 1 & 0 \\ 0 & -2 & 0 & 0 & 0 & 2 \end{bmatrix}$$

Element 2:

$$\mathbf{k}_{e2} = \int_{A_e} h \mathbf{B}_{e2}^T \mathbf{C} \mathbf{B}_{e2} dA = h \int_{A_e} \underbrace{\mathbf{B}_{e2}^T \mathbf{C} \mathbf{B}_{e2}}_{\text{constant}} dA = A_2 h \mathbf{B}_{e2}^T \mathbf{C} \mathbf{B}_{e2} = \frac{l^2}{2} h \mathbf{B}_{e2}^T \mathbf{C} \mathbf{B}_{e2}$$

$$\Rightarrow \mathbf{B}_{e2} = \frac{l}{2Ae,2} \begin{bmatrix} 0 & 0 & 1 & 0 & -1 & 0 \\ 0 & -1 & 0 & 1 & 0 & 0 \\ -1 & 0 & 1 & 1 & 1 & -1 \end{bmatrix} = \frac{1}{l} \begin{bmatrix} 0 & 0 & 1 & 0 & -1 & 0 \\ 0 & -1 & 0 & 1 & 0 & 0 \\ -1 & 0 & 1 & 1 & 1 & -1 \end{bmatrix}$$

$$\mathbf{k}_{e2} = \frac{Eh}{4} \begin{bmatrix} 1 & 0 & -1 & -1 & 0 & 1 \\ 0 & 2 & 0 & -2 & 0 & 0 \\ -1 & 0 & 3 & 1 & -2 & -1 \\ -1 & -2 & 1 & 3 & 0 & -1 \\ 0 & 0 & -2 & 0 & 2 & 0 \\ 1 & 0 & -1 & -1 & 0 & 1 \end{bmatrix}$$

Assembling:

$$k_{e1} = \frac{Eh}{4} \begin{bmatrix} 3 & 1 & -2 & -1 & -1 & 0 \\ 1 & 3 & 0 & -1 & -1 & -2 \\ -2 & 0 & 2 & 0 & 0 & 0 \\ -1 & -1 & 0 & 1 & 1 & 0 \\ -1 & -1 & 0 & 1 & 1 & 0 \\ 0 & -2 & 0 & 0 & 0 & 2 \end{bmatrix} \begin{matrix} 1x \\ 1y \\ 2x \\ 2y \\ 4x \\ 4y \end{matrix}$$

$$k_{e2} = \frac{Eh}{4} \begin{bmatrix} 1 & 0 & -1 & -1 & 0 & 1 \\ 0 & 2 & 0 & -2 & 0 & 0 \\ -1 & 0 & 3 & 1 & -2 & -1 \\ -1 & -2 & 1 & 3 & 0 & -1 \\ 0 & 0 & -2 & 0 & 2 & 0 \\ 1 & 0 & -1 & -1 & 0 & 1 \end{bmatrix} \begin{matrix} 2x \\ 2y \\ 3x \\ 3y \\ 4x \\ 4y \end{matrix}$$

$$\Rightarrow K = \frac{Eh}{4} \begin{bmatrix} +3 & +1 & -2 & -1 & 0 & 0 & -1 & 0 \\ +1 & +3 & 0 & -1 & 0 & 0 & -1 & -2 \\ -2 & 0 & +2 & +1 & 0 & -1 & -1 & 0 \\ -1 & -1 & 0 & +1 & +2 & 0 & -2 & +1 \\ 0 & 0 & -1 & 0 & +3 & +1 & -2 & -1 \\ 0 & 0 & -1 & -2 & +1 & +3 & 0 & -1 \\ -1 & -1 & 0 & +1 & -2 & 0 & +1 & +2 \\ 0 & -2 & +1 & +1 & 0 & -1 & -1 & 0 & +2 & +1 \end{bmatrix} \begin{matrix} 1x \\ 1y \\ 2x \\ 2y \\ 3x \\ 3y \\ 4x \\ 4y \end{matrix}$$

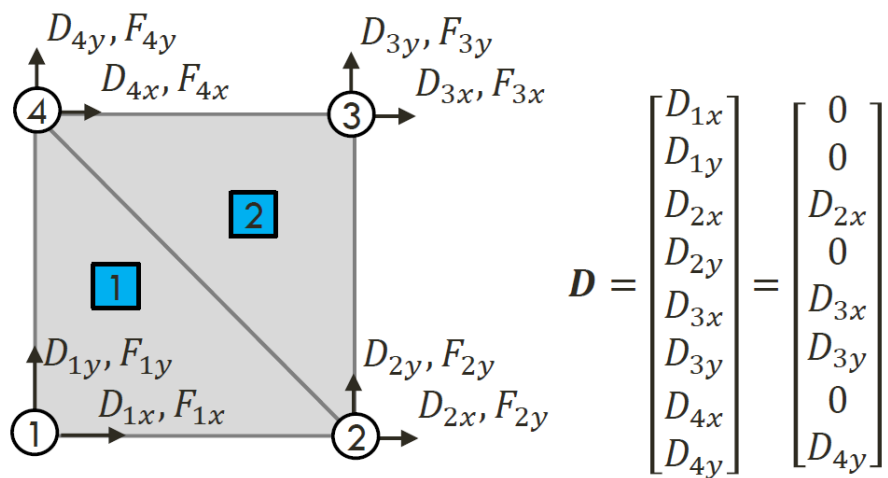
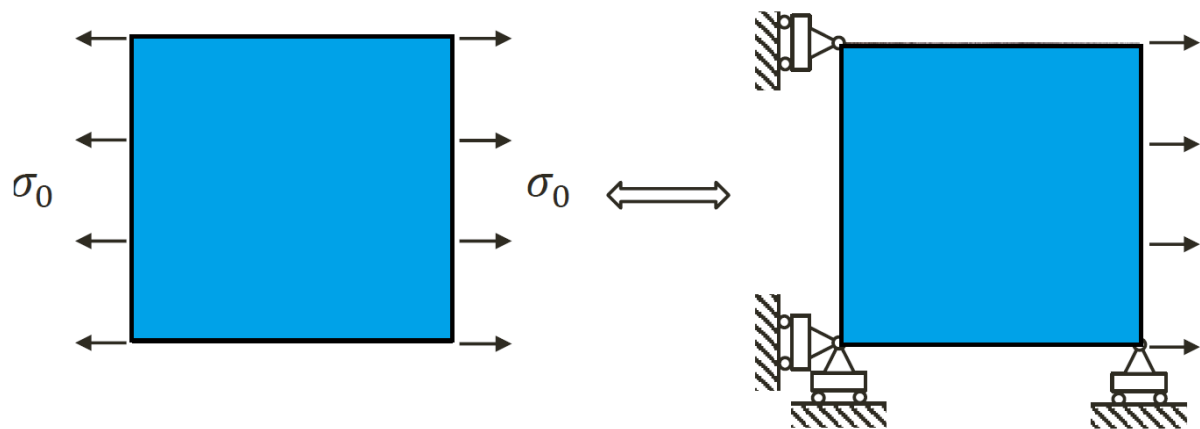
$$\Leftrightarrow K = \frac{Eh}{4} \begin{bmatrix} 3 & 1 & -2 & -1 & 0 & 0 & -1 & 0 \\ 1 & 3 & 0 & -1 & 0 & 0 & -1 & -2 \\ -2 & 0 & 3 & 0 & -1 & -1 & 0 & 1 \\ -1 & -1 & 0 & 3 & 0 & -2 & 1 & 0 \\ 0 & 0 & -1 & 0 & 3 & 1 & -2 & -1 \\ 0 & 0 & -1 & -2 & 1 & 3 & 0 & -1 \\ -1 & -1 & 0 & 1 & -2 & 0 & 3 & 0 \\ 0 & -2 & 1 & 0 & -1 & -1 & 0 & 3 \end{bmatrix}$$

Vector of forces

The expression of the element force vector for the CST comes from

$$\left[\int_{V_e} \mathbf{B}^T \mathbf{C} \mathbf{B} dV \right] \mathbf{d}_e = \underbrace{\left[\int_{S_e} \mathbf{N}^T \mathbf{t} dS + \int_{V_e} \mathbf{N}^T \mathbf{K}_b dV \right]}_{\mathbf{F}_e}$$

LOAD CASE 1:



Vector of Nodal Forces:

$$\Rightarrow \mathbf{F}_{nod}^T = [R_{1x} \quad R_{1y} \quad 0 \quad R_{2y} \quad 0 \quad 0 \quad R_{4x} \quad 0]$$

Vector of Superficial Forces:

(they act only on Element 2)

$$\mathbf{F}_{s,2} = \int_{s_e} \mathbf{N}^T \mathbf{t} dS = \int_{s_e} \begin{bmatrix} N_1 \\ 0 \\ N_2 \\ 0 \\ N_3 \\ 0 \\ 0 \\ N_3 \end{bmatrix} \begin{bmatrix} t_x \\ t_y \end{bmatrix} dS = \int_0^l \begin{bmatrix} N_1 \\ 0 \\ N_2 \\ 0 \\ N_3 \\ 0 \\ 0 \\ N_3 \end{bmatrix} \begin{bmatrix} \sigma_0 \\ 0 \end{bmatrix} \cdot h \cdot dy = h \int_0^l \begin{bmatrix} N_1 \sigma_0 \\ 0 \\ N_2 \sigma_0 \\ 0 \\ N_3 \sigma_0 \\ 0 \\ 0 \\ 0 \end{bmatrix} dy = h \sigma_0 \int_0^l \begin{bmatrix} 1 - \frac{y}{l} \\ 0 \\ \frac{x}{l} + \frac{y}{l} - 1 \\ 0 \\ 1 - \frac{x}{l} \\ 0 \\ 0 \\ 0 \end{bmatrix} dy$$

$$\Rightarrow \mathbf{F}_{s,2}^T = \frac{\sigma_0 h l}{2} [1 \quad 0 \quad 1 \quad 0 \quad 0 \quad 0]$$

No Body Forces

$$\mathbf{F} = \mathbf{F}_{nod} + \mathbf{F}_s = \begin{bmatrix} R_{1x} \\ R_{1y} \\ 0 \\ R_{2y} \\ 0 \\ 0 \\ R_{4x} \\ 0 \end{bmatrix} + \frac{\sigma_0 hl}{2} \begin{bmatrix} 0 \\ 0 \\ 1 \\ 0 \\ 1 \\ 0 \\ 0 \\ 0 \end{bmatrix} = \begin{bmatrix} R_{1x} \\ R_{1y} \\ \frac{\sigma_0 hl}{2} \\ R_{2y} \\ \frac{\sigma_0 hl}{2} \\ 0 \\ R_{4x} \\ 0 \end{bmatrix}$$

Solving the system:

$$\mathbf{F} = \mathbf{K}\mathbf{D} \quad \Rightarrow \mathbf{F}_{red} = \mathbf{K}_{red}\mathbf{D}_{red}$$

$$\begin{bmatrix} R_{1x} \\ R_{1y} \\ \frac{\sigma_0 hl}{2} \\ R_{2y} \\ \frac{\sigma_0 hl}{2} \\ 0 \\ R_{4x} \\ 0 \end{bmatrix} = \frac{Eh}{4} \begin{bmatrix} 3 & 1 & -2 & -1 & 0 & 0 & -1 & 0 \\ 1 & 3 & 0 & -1 & 0 & 0 & -1 & -2 \\ -2 & 0 & 3 & 0 & -1 & -1 & 0 & 1 \\ -1 & -1 & 0 & 3 & 0 & -2 & 1 & 0 \\ 0 & 0 & -1 & 0 & 3 & 1 & -2 & -1 \\ 0 & 0 & -1 & -2 & 1 & 3 & 0 & -1 \\ -1 & -1 & 0 & 1 & -2 & 0 & 3 & 0 \\ 0 & -2 & 1 & 0 & -1 & -1 & 0 & 3 \end{bmatrix} \begin{bmatrix} 0 \\ 0 \\ D_{2x} \\ 0 \\ D_{3x} \\ D_{3y} \\ 0 \\ D_{4y} \end{bmatrix}$$

$$\begin{bmatrix} \frac{\sigma_0 hl}{2} \\ \frac{\sigma_0 hl}{2} \\ 0 \\ 0 \end{bmatrix} = \frac{Eh}{4} \begin{bmatrix} 3 & -1 & -1 & 1 \\ -1 & 3 & 1 & -1 \\ -1 & 1 & 3 & -1 \\ 1 & -1 & -1 & 3 \end{bmatrix} \begin{bmatrix} D_{2x} \\ D_{3x} \\ D_{3y} \\ D_{4y} \end{bmatrix}$$

Displacements:

$$\Rightarrow \begin{bmatrix} D_{2x} \\ D_{3x} \\ D_{3y} \\ D_{4y} \end{bmatrix} = \frac{\sigma_0 l}{E} \begin{bmatrix} 1 \\ 1 \\ 0 \\ 0 \end{bmatrix}$$

Forces:

$$\begin{bmatrix} R_{1x} \\ R_{1y} \\ \frac{\sigma_0 hl}{2} \\ R_{2y} \\ \frac{\sigma_0 hl}{2} \\ 0 \\ R_{4x} \\ 0 \end{bmatrix} = \frac{Eh}{4} \begin{bmatrix} 3 & 1 & -2 & -1 & 0 & 0 & -1 & 0 \\ 1 & 3 & 0 & -1 & 0 & 0 & -1 & -2 \\ -2 & 0 & 3 & 0 & -1 & -1 & 0 & 1 \\ -1 & -1 & 0 & 3 & 0 & -2 & 1 & 0 \\ 0 & 0 & -1 & 0 & 3 & 1 & -2 & -1 \\ 0 & 0 & -1 & -2 & 1 & 3 & 0 & -1 \\ -1 & -1 & 0 & 1 & -2 & 0 & 3 & 0 \\ 0 & -2 & 1 & 0 & -1 & -1 & 0 & 3 \end{bmatrix} \begin{bmatrix} 0 \\ 0 \\ \frac{\sigma_0 l}{E} \\ 0 \\ \frac{\sigma_0 l}{E} \\ 0 \\ 0 \\ 0 \end{bmatrix} \Rightarrow \begin{bmatrix} R_{1x} \\ R_{1y} \\ R_{2y} \\ R_{4x} \end{bmatrix} = \frac{\sigma_0 hl}{2} \begin{bmatrix} -1 \\ 0 \\ 0 \\ -1 \end{bmatrix}$$

Post- Processing

Element 1

$$\sigma_1 = E \underbrace{\begin{bmatrix} 1 & 0 & 0 \\ 0 & 1 & 0 \\ 0 & 0 & \frac{1}{2} \end{bmatrix}}_{c_1|v=0} \underbrace{\frac{1}{l} \begin{bmatrix} -1 & 0 & 1 & 0 & 0 & 0 \\ 0 & -1 & 0 & 0 & 0 & 1 \\ -1 & -1 & 0 & 1 & 1 & 0 \end{bmatrix}}_{B_1} \underbrace{\frac{\sigma_0 l}{E} \begin{bmatrix} 0 \\ 0 \\ 1 \\ 0 \\ 0 \\ 0 \end{bmatrix}}_{d_{e,1}} \Leftrightarrow \sigma_1 = \begin{bmatrix} \sigma_x \\ \sigma_y \\ \tau_{xy} \end{bmatrix} = \begin{bmatrix} \sigma_0 \\ 0 \\ 0 \end{bmatrix}$$

D 1x
D 1y
D 2x
D 2y
D 4x
D 4y

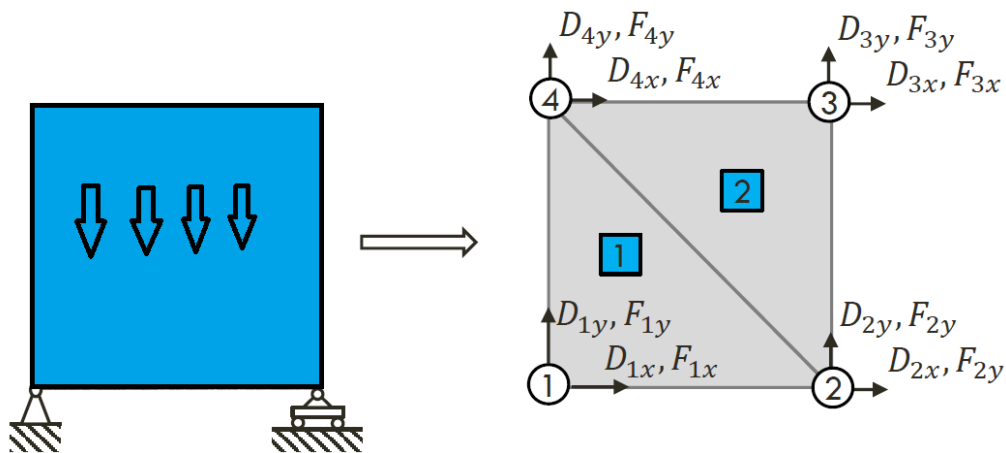
Element 2

$$\sigma_2 = E \underbrace{\begin{bmatrix} 1 & 0 & 0 \\ 0 & 1 & 0 \\ 0 & 0 & \frac{1}{2} \end{bmatrix}}_{c_2|v=0} \underbrace{\frac{1}{l} \begin{bmatrix} 0 & 0 & 1 & 0 & -1 & 0 \\ 0 & -1 & 0 & 1 & 0 & 0 \\ -1 & 0 & 1 & 1 & 1 & -1 \end{bmatrix}}_{B_2} \underbrace{\frac{\sigma_0 l}{E} \begin{bmatrix} 1 \\ 0 \\ 1 \\ 0 \\ 0 \\ 0 \end{bmatrix}}_{d_{e,2}} \Leftrightarrow \sigma_2 = \begin{bmatrix} \sigma_x \\ \sigma_y \\ \tau_{xy} \end{bmatrix} = \begin{bmatrix} \sigma_0 \\ 0 \\ 0 \end{bmatrix}$$

D 2x
D 2y
D 3x
D 3y
D 4x
D 4y

We only have tension in the X direction. OK !

LOAD CASE 3:



Force Vector

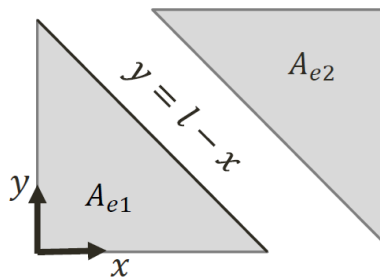
Nodal Force Vector

$$\Rightarrow \mathbf{F}_{nod}^T = [R_{1x} \quad R_{1y} \quad 0 \quad R_{2y} \quad 0 \quad 0 \quad 0 \quad 0]$$

Body Force Vector:

$$\mathbf{F}_{b,e} = \int_{V_e} \mathbf{N}^T \mathbf{K}_b dV$$

$$\Rightarrow \mathbf{F}_{b,e} = \int_{V_e} \begin{bmatrix} N_1 & 0 \\ 0 & N_1 \\ N_2 & 0 \\ 0 & N_2 \\ N_3 & 0 \\ 0 & N_3 \end{bmatrix} \begin{bmatrix} K_x \\ K_y \end{bmatrix} dV = \int_{A_e} \begin{bmatrix} N_1 & 0 \\ 0 & N_1 \\ N_2 & 0 \\ 0 & N_2 \\ N_3 & 0 \\ 0 & N_3 \end{bmatrix} \begin{bmatrix} K_x \\ K_y \end{bmatrix} h dA = \iint \begin{bmatrix} N_1 & 0 \\ 0 & N_1 \\ N_2 & 0 \\ 0 & N_2 \\ N_3 & 0 \\ 0 & N_3 \end{bmatrix} \begin{bmatrix} K_x \\ K_y \end{bmatrix} h \cdot dx \cdot dy$$



$$\Rightarrow \mathbf{F}_{b,1} = \int_0^l \int_0^{l-x} \begin{bmatrix} N_1 & 0 \\ 0 & N_1 \\ N_2 & 0 \\ 0 & N_2 \\ N_3 & 0 \\ 0 & N_3 \end{bmatrix} \begin{bmatrix} 0 \\ -\rho g \end{bmatrix} h \cdot dy \cdot dx = (\dots)$$

$$\Rightarrow \mathbf{F}_{b,2} = \int_0^l \int_{l-x}^l \begin{bmatrix} N_1 & 0 \\ 0 & N_1 \\ N_2 & 0 \\ 0 & N_2 \\ N_3 & 0 \\ 0 & N_3 \end{bmatrix} \begin{bmatrix} 0 \\ -\rho g \end{bmatrix} h \cdot dy \cdot dx = (\dots)$$

Element 1:

$$\Rightarrow \mathbf{F}_{b,1} = \int_0^l \int_0^{l-x} -\rho g \begin{bmatrix} 1 - \frac{x}{l} - \frac{y}{l} \\ 0 \\ \frac{x}{l} \\ 0 \\ \frac{y}{l} \\ 0 \end{bmatrix} h \cdot dy \cdot dx = -\frac{\rho g h}{2l} \int_0^l \begin{bmatrix} l^2 - 2lx + x^2 \\ 0 \\ 2lx - 2x^2 \\ 0 \\ l^2 - 2lx + x^2 \end{bmatrix} dx = -\frac{\rho g h l^2}{6} \begin{bmatrix} 0 \\ 1 \\ 0 \\ 1 \\ 0 \\ 1 \end{bmatrix} \begin{matrix} 1x \\ 1y \\ 2x \\ 2y \\ 4x \\ 4y \end{matrix}$$

Element 2:

$$\Rightarrow \mathbf{F}_{b,2} = \int_0^l \int_{l-x}^l -\rho g \begin{bmatrix} 0 \\ 1 - \frac{y}{l} \\ \frac{x}{l} + \frac{y}{l} - 1 \\ 0 \\ 1 - \frac{x}{l} \\ 0 \end{bmatrix} h \cdot dy \cdot dx = \dots = -\frac{\rho g h l^2}{6} \begin{bmatrix} 0 \\ 1 \\ 0 \\ 1 \\ 0 \\ 1 \end{bmatrix} \begin{matrix} 2x \\ 2y \\ 3x \\ 3y \\ 4x \\ 4y \end{matrix}$$

$$F = F_{nod} + F_{b,1} + F_{b,2} = \begin{bmatrix} R_{1x} \\ R_{1y} \\ 0 \\ R_{2y} \\ 0 \\ 0 \\ 0 \\ 0 \end{bmatrix} - \frac{\rho g h l^2}{6} \begin{bmatrix} 0 \\ 1 \\ 0 \\ 1 \\ 0 \\ 0 \\ 0 \\ 1 \end{bmatrix} - \frac{\rho g h l^2}{6} \begin{bmatrix} 0 \\ 0 \\ 0 \\ 1 \\ 0 \\ 1 \\ 0 \\ 1 \end{bmatrix} = \begin{bmatrix} R_{1x} \\ R_{1y} - \frac{\rho g h l^2}{6} \\ 0 \\ R_{2y} - \frac{\rho g h l^2}{3} \\ 0 \\ -\frac{\rho g h l^2}{6} \\ 0 \\ -\frac{\rho g h l^2}{3} \end{bmatrix}$$

Solving the system.

$$\begin{bmatrix} R_{1x} \\ R_{1y} - \frac{\rho g h l^2}{6} \\ 0 \\ R_{2y} - \frac{\rho g h l^2}{3} \\ 0 \\ -\frac{\rho g h l^2}{6} \\ 0 \\ -\frac{\rho g h l^2}{3} \end{bmatrix} = \frac{Eh}{4} \begin{bmatrix} 3 & 1 & -2 & -1 & 0 & 0 & -1 & 0 \\ 1 & 3 & 0 & -1 & 0 & 0 & -1 & -2 \\ -2 & 0 & 3 & 0 & -1 & -1 & 0 & 1 \\ -1 & -1 & 0 & 3 & 0 & -2 & 1 & 0 \\ 0 & 0 & -1 & 0 & 3 & 1 & -2 & -1 \\ 0 & 0 & -1 & -2 & 1 & 3 & 0 & -1 \\ -1 & -1 & 0 & 1 & -2 & 0 & 3 & 0 \\ 0 & -2 & 1 & 0 & -1 & -1 & 0 & 3 \end{bmatrix} \begin{bmatrix} 0 \\ 0 \\ D_{2x} \\ 0 \\ D_{3x} \\ D_{3y} \\ D_{4x} \\ D_{4y} \end{bmatrix}$$

$$\Rightarrow F_{red} = K_{red} D_{red} \Leftrightarrow$$

$$\begin{bmatrix} 0 \\ 0 \\ -\frac{\rho g h l^2}{6} \\ 0 \\ -\frac{\rho g h l^2}{3} \end{bmatrix} = \frac{Eh}{4} \begin{bmatrix} 3 & -1 & -1 & 0 & 1 \\ -1 & 3 & 1 & -2 & -1 \\ -1 & 1 & 3 & 0 & -1 \\ 0 & -2 & 0 & 3 & 0 \\ 1 & -1 & -1 & 0 & 3 \end{bmatrix} \begin{bmatrix} D_{2x} \\ D_{3x} \\ D_{3y} \\ D_{4x} \\ D_{4y} \end{bmatrix} \Rightarrow \begin{bmatrix} D_{2x} \\ D_{3x} \\ D_{3y} \\ D_{4x} \\ D_{4y} \end{bmatrix} = \frac{\rho g l^2}{24E} \begin{bmatrix} 1 \\ -3 \\ -9 \\ -2 \\ -15 \end{bmatrix}$$

Finding the reaction forces

$$\begin{bmatrix} R_{1x} \\ R_{1y} - \frac{\rho g h l^2}{6} \\ 0 \\ R_{2y} - \frac{\rho g h l^2}{3} \\ 0 \\ -\frac{\rho g h l^2}{6} \\ 0 \\ -\frac{\rho g h l^2}{3} \end{bmatrix} = \frac{Eh}{4} \begin{bmatrix} 3 & 1 & -2 & -1 & 0 & 0 & -1 & 0 \\ 1 & 3 & 0 & -1 & 0 & 0 & -1 & -2 \\ -2 & 0 & 3 & 0 & -1 & -1 & 0 & 1 \\ -1 & -1 & 0 & 3 & 0 & -2 & 1 & 0 \\ 0 & 0 & -1 & 0 & 3 & 1 & -2 & -1 \\ 0 & 0 & -1 & -2 & 1 & 3 & 0 & -1 \\ -1 & -1 & 0 & 1 & -2 & 0 & 3 & 0 \\ 0 & -2 & 1 & 0 & -1 & -1 & 0 & 3 \end{bmatrix} \frac{\rho g l^2}{24E} \begin{bmatrix} 0 \\ 0 \\ 1 \\ 0 \\ -3 \\ -9 \\ -2 \\ -15 \end{bmatrix}$$

$$\Rightarrow \begin{bmatrix} F_{1x} \\ F_{1y} \\ F_{2x} \\ F_{2y} \\ F_{3x} \\ F_{3y} \\ F_{4x} \\ F_{4y} \end{bmatrix} = \begin{bmatrix} R_{1x} \\ R_{1y} - \frac{\rho g h l^2}{6} \\ 0 \\ R_{2y} - \frac{\rho g h l^2}{3} \\ 0 \\ -\frac{\rho g h l^2}{6} \\ 0 \\ -\frac{\rho g h l^2}{3} \end{bmatrix} = \begin{bmatrix} 0 \\ \frac{\rho g h l^2}{3} \\ 0 \\ \frac{\rho g h l^2}{6} \\ 0 \\ \frac{\rho g h l^2}{6} \\ 0 \\ -\frac{\rho g h l^2}{3} \end{bmatrix} \Rightarrow \begin{bmatrix} R_{1x} \\ R_{1y} \\ R_{2y} \end{bmatrix} = \begin{bmatrix} \frac{\rho g h l^2}{2} \\ \frac{\rho g h l^2}{3} \\ \frac{\rho g h l^2}{2} \end{bmatrix}$$

Post-Processing

$$\sigma = CBD_e$$

$$\boldsymbol{\sigma}_1 = E \underbrace{\begin{bmatrix} 1 & 0 & 0 \\ 0 & 1 & 0 \\ 0 & 0 & \frac{1}{2} \end{bmatrix}}_{\mathcal{C}|_{v=0}} \underbrace{\frac{1}{l} \begin{bmatrix} -1 & 0 & 1 & 0 & 0 & 0 \\ 0 & -1 & 0 & 0 & 0 & 1 \\ -1 & -1 & 1 & 1 & 1 & 0 \end{bmatrix}}_{\mathbf{B}_1} \underbrace{\frac{\rho g l^2}{24E} \begin{bmatrix} 0 \\ 0 \\ 1 \\ 0 \\ -2 \\ -15 \end{bmatrix}}_{\mathbf{d}_{e,1}} \Leftrightarrow \boldsymbol{\sigma}_1 = \begin{bmatrix} \sigma_x \\ \sigma_y \\ \tau_{xy} \end{bmatrix} = \frac{\rho g l^2}{24} \begin{bmatrix} 1 \\ -15 \\ -1 \end{bmatrix}$$

$$\boldsymbol{\sigma}_2 = E \underbrace{\begin{bmatrix} 1 & 0 & 0 \\ 0 & 1 & 0 \\ 0 & 0 & \frac{1}{2} \end{bmatrix}}_{\mathcal{C}|_{v=0}} \underbrace{\frac{1}{l} \begin{bmatrix} 0 & 0 & 1 & 0 & -1 & 0 \\ 0 & -1 & 0 & 1 & 0 & 0 \\ -1 & 0 & 1 & 1 & 0 & -1 \end{bmatrix}}_{\mathbf{B}_2} \underbrace{\frac{\rho g l^2}{24E} \begin{bmatrix} 1 \\ 0 \\ -3 \\ -9 \\ -2 \\ -15 \end{bmatrix}}_{\mathbf{d}_{e,2}} \Leftrightarrow \boldsymbol{\sigma}_1 = \begin{bmatrix} \sigma_x \\ \sigma_y \\ \tau_{xy} \end{bmatrix} = \frac{\rho g l^2}{24} \begin{bmatrix} -1 \\ -9 \\ 1 \end{bmatrix}$$